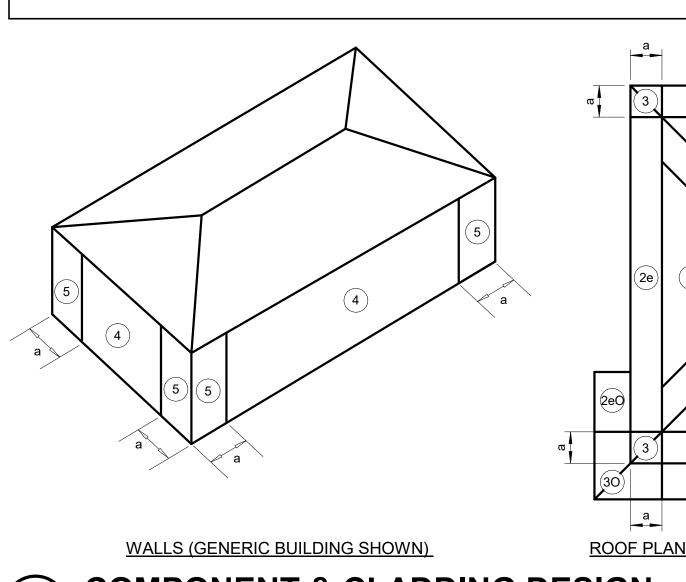
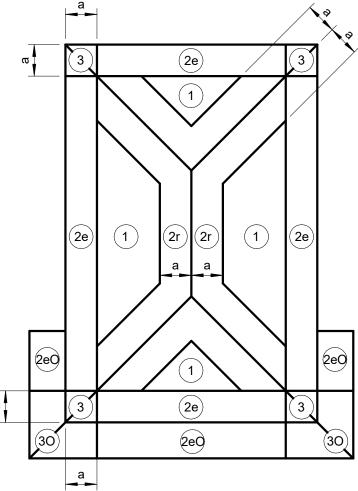
A MOMER OF POLINT CAP <							DESIGN CRITERIA
B B B B B C	STRUCTURAL ABBREVIATIONS					DESIGN PER 2023 FLORIDA BUILDING CODE, UNLESS OTHERWISE NOTED.	
P BULLORE ALD Constraint	#						
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G AT GT GROCE TRUGS SPACE SOLITION STANDARD MULTINITY (MILLING) MULTINI							
Math Automation Control SPCCT							ROOF
ALUMANN ALUMANN Hold Hondow Hold Hondow R R Charles ASULANCE SULANCE S	<u>w</u>	AI	GI	GIRDER TRUSS			
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ARCH ARCHITECTURE HBA HEADES JUD ANCHOR STIFF BUT STUPPER STIFF BUT STUPPER MADD ANCHOR Last							NOMINAL WIND SPEED. (ASCE 7-22)
ASSY ASSEMULY IF IP INDEFACE STLE TELL BREAK BREAK INT							MEAN ROOF HEIGHT 13.5 FT
B CTTOLUT F NOT TOLUT F NOT TOLUT STRUE DEVENDER <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>							
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BOTTOM BOTTOM JAT JOINT TUT MALE TO DO Construction			INT	INTERIOR			DIRECTIONALITY FACTOR (Kd) 0.85
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CTOC CENTER TO CENTER LG LONO THRO THR	BRG	BEARING	IR				GLAZING IS NOT REQUIRED.
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FDN FOUNDATION REQ'D REQUIRED FF FINISH FLOOR RET RETAINING 3" FLR FLOOR REV REVISION 2" FROM TOP FS FAR SIDE ALL REINFORCING SHALL BE HELD SECURELY IN POSITION WITH STANDARD ACCESSORIES ALL REINFORCING SHALL BE HELD SECURELY IN POSITION WITH STANDARD ACCESSORIES FTG FOOTING IN CONFORMANCE WITH CRSI MANUAL OF STANDARD PRACTICE AND ACI 318 DURING							
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FLR FLOOR REV REVISION FS FAR SIDE FTG FOOTING							FOOTINGS
FSFAR SIDEFTGFOOTINGALL REINFORCING SHALL BE HELD SECURELY IN POSITION WITH STANDARD ACCESSORIESIN CONFORMANCE WITH CRSI MANUAL OF STANDARD PRACTICE AND ACI 318 DURING							SLABS ON GRADE
FTG FOOTING IN CONFORMANCE WITH CRSI MANUAL OF STANDARD PRACTICE AND ACI 318 DURING			REV	REVISIUN			ALL REINFORCING SHALL BE HELD SECURELY IN POSITION WITH STANDARD ACCESSORIES
							THE PLACING OF THE CONCRETE.

COMPONENT & CLADDING DESIGN WIND PRESSURES (PSF)								
ZONE	10 SF	100 SF	200 SF	500 SF				
NEGATIVE ZONE 1	-55.5	-33.9	-27.5	-27.5				
NEGATIVE ZONE 2	-72.3	-48.6	-41.5	-41.5				
NEGATIVE ZONE 3	-77.9	-52.1	-44.3	-44.3				
POSITIVE ALL ZONES	24.7	16.0	16.0	16.0				
OVERHANG 3 & 4	-103.7	-72.9	-63.6	-61.6				
OVERHANG 3 & 5	-112.1	-76.3	-65.6	-61.6				
		WALL			NOTES: 1. TABLE PRESSURES ARE FOR THE SQUARE FOOT (SF)			
ZONE	10 SF	100 SF	200 SF	500 SF	TF	TRIBUTARY AREA SHOWN. FOR OTHER TRIBUTARY AREAS, LINEARLY INTERPOLATE BETWEEN VALUE		
NEGATIVE ZONE 4	-35.9	-30.9	-29.4	-27.5	SI	POSITIVE PRESSURES ACT TOWARD THE BUILDING.		
NEGATIVE ZONE 5	-44.3	-34.4	-31.4	-27.5	N	IEGATIVE PRESSURES ACT AWAY FROM THE		
POSITIVE ZONE 4 & 5	33.1	28.1	26.6	24.7	3. SI	SEE DIAGRAMS FOR LOCATION OF ZONES. PRESSURES SHOWN ARE ULTIMATE PRESSURES.		





ROOF PLAN (GENERIC BUILDING SHOWN)

a=5'-8"

COMPONENT & CLADDING DESIGN

UNLESS OTHERWISE NOTED, SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE AS

FOLLOWS: WELDED WIRE FABRIC WIRE SPACING PLUS 6" REINFORCING BARS SEE SCHED 6/S2.1 ALL HOOKS IN REINFORCING BARS SHALL BE AN ACI STANDARD HOOK, UNLESS OTHERWISE

FASTENERS

NOTED

EXPANSION BOLTS SHALL BE HILTI KWIK BOLT 3, SIMPSON STRONG-TIE STRONG-BOLT2, DEWALT POWER-STUD+ SD1 OR APPROVED EQUAL, UON. EMBEDMENT DEPTH INTO CONCRETE OR SOLID GROUTED MASONRY SHALL BE AT LEAST 7 TIMES THE BOLT DIAMETER. UON. CLEAN HOLE AND INSTALL IN STRICT ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

SCREW ANCHORS SHALL BE HILTI KWIK HUS-EZ, SIMPSON STRONG-TIE TITEN HD, DEWALT SCREW-BOLT+ OR APPROVED EQUAL, UON. EMBEDMENT IN CONCRETE OR SOLID GROUTED MASONRY SHALL BE AT LEAST 9 TIMES THE BOLT DIAMETER, UON. CLEAN HOLE AND INSTALL IN STRICT ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

POWER ACTUATED FASTENERS (PAF) SHALL BE 0.157" DIAMETER HILTI X-U, SIMPSON STRONG-TIE PDPA. DEWALT CSI PIN OR EQUAL, UON, EMBED MINIMUM1 1/4" INTO CONCRETE AND CMU. UON. DO NOT PLACE WITHIN 1" OF CMU MORTAR JOINT. PAF SHALL COMPLETELY PENETRATE STRUCTURAL STEEL.

ADHESIVE ANCHORING (EPOXY):

 ADHESIVE ANCHORING FOR CONCRETE SHALL BE HILTI RE-500 V3 CARTRIDGE SYSTEM, SIMPSON STRONG-TIE SET-3G. DEWALT PURE 110+ (OR EQUIVALENT ACRYLIC AC200+, HY 200, OR ATXP) OR APPROVED EQUAL, UON. EMBEDMENT DEPTH SHALL BE AT LEAST 12 TIMES THE INSERT DIAMETER, UON. HOLE DIAMETER SHALL BE NO GREATER THAN RECOMMENDED BY MANUFACTURER. THE HOLE SHALL BE CLEANED PER MANUFACTURER'S RECOMMENDATIONS BY BRUSHING OUT WITH WIRE BOTTLE BRUSH AND BLOWN OUT WITH AIR USING A COMPRESSOR WITH A FUNCTIONAL OIL TRAP (EXCEPT WHERE PERMITTED WHEN USING A DUST EXTRACTION SYSTEM IN ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS).

GENERAL - ANCHORS SHALL MEET THE REQUIREMENTS OF ACI 355.4. INSTALLATION SHALL BE IN STRICT ACCORDANCE WITH MANUFACTURER'S PRINTED INSTRUCTIONS AND PERFORMED BY AN INSTALLER TRAINED BY THE MANUFACTURER. INSTALLATION OF ADHESIVE ANCHORS HORIZONTALLY OR UPWARDLY, WHICH SUPPORT SUSTAINED TENSION LOADS SHALL BE PERFORMED BY A CERTIFIED ADHESIVE ANCHOR INSTALLER IN ACCORDANCE WITH ACI318 AND CONTINUOUSLY INSPECTED PER ACI318. PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL.

 CAPACITIES - UON, DESIGN BOND STRENGTH OF ANCHORS HAVE BEEN BASED ON CRACKED CONCRETE, ACI 355.4 TEMPERATURE CATEGORY B, AND INSTALLATIONS INTO DRY HOLES DRILLED WITH A ROTARY IMPACT DRILL OR ROCK DRILL INTO CONCRETE THAT HAS CURED AT LEAST 21 DAYS AND HAS A CONCRETE TEMPERATURE OF AT LEAST 50 DEGREES F AT TIME OF ANCHOR INSTALLATION.

<u>MASONRY</u>

ALL LOAD BEARING WALLS AND EXTERIOR WALLS SHALL BE COMPOSED OF ASTM C90 HOLLOW CONCRETE MASONRY UNITS WITH ASTM C270 TYPE "S" MORTAR. GROUT SHALL CONFORM TO THE REQUIREMENTS OF ASTM C476 AND HAVE A COMPRESSIVE STRENGTH OF 3000 PSI.

ALL EXTERIOR CMU WALLS SHALL BE REINFORCED FULL HEIGHT IN A GROUT FILLED CELL WITH (1) #5 AT:

- EACH CORNER, WALL ENDS, WALL INTERSECTIONS EACH SIDE OF CONTROL JOINTS AND
- AT A MAXIMUM SPACING OF 4'-0" OC, UON
- SEE DETAIL 1/S4.1 FOR TYPICAL REINFORCING AT WALL OPENINGS. AT BEAM & JOIST GIRDER BEARING LOCATIONS ADD REINFORCING AS SHOWN IN PLAN.

LAPPED BARS SHALL BE SECURED WITH WIRE TIES OR OTHER MEANS TO ENSURE THAT THE BAR IS NOT DISPLACED DURING GROUT PLACEMENT OUTSIDE THE TOLERANCES ESTABLISHED BY ACI 530. LAP BARS WITH THE FOLLOWING MINIMUM LENGTH.

	BARS CTR'D
BAR SIZE	<u>8" CMU</u>
#3	16"
#4	21"
#5	26"

GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT IN 5'-0" MAXIMUM LIFTS. DO NOT BEGIN PLACEMENT OF GROUT UNTIL ALIGNMENT OF CELLS ARE INSPECTED AND APPROVED.

FILL ALL CELLS BELOW FINISHED GRADE.

PROVIDE HORIZONTAL JOINT REINFORCEMENT IN WALLS AT 16" OC VERTICALLY, UON. IN ADDITION, INSTALL JOINT REINFORCING IN THE FIRST TWO MORTAR JOINTS ABOVE & BELOW OPENINGS, EXTENDING AT LEAST 24 INCHES BEYOND THE OPENING. PROVIDE HORIZONTAL JOINT REINFORCEMENT IN PARAPETS AND FREE STANDING WALLS AND 8" OC VERTICALLY. LAP JOINT REINFORCEMENT 6" MINIMUM. HORIZONTAL REINFORCING SHALL CONSIST OF AT LEAST TWO W1.7 WIRES OR GREATER.

SEE ARCHITECTURAL DRAWING FOR EXPANSION OR CONTROL JOINTS. IF NOT SHOWN, LOCATE VERTICAL CONTROL JOINTS AT 25'-0" OC MAXIMUM, BUT NOT LESS THAN 2'-0" FROM A JOIST OR BEAM BEARING PLATE. AT BUILDING CORNERS, PROVIDE ONE JOINT IN ONE OF THE TWO WALL SIDES NO MORE THAN 5'-0" FROM THE BUILDING CORNER. HORIZONTAL REINFORCING SHALL CONSIST OF W1.7 JOINT REINFORCEMENT OR GREATER.

ALL PRECAST OR POURED LINTELS SHALL BE REINFORCED WITH (2) #4 TOP & BOTTOM WITH #3 TIES @ 12" AS A MINIMUM AND HAVE A MINIMUM MASONRY END BEARING OF 8".

BOND/TIE BEAM REINFORCEMENT SHALL BE CONTINUOUS ACROSS CONTROL JOINTS.

16" U-BLOCK OR BOND BEAM SHALL CONSIST OF TWO 8" KNOCK-OUT BLOCKS.

MASONRY WORK SHALL BE INSPECTED IN ACCORDANCE WITH TMS 402 QUALITY ASSURANCE LEVEL B

<u>WOOD</u>

CLIPS, CONNECTIONS, HANGERS, HOLD-DOWNS, ETC. SHOWN ON THESE DRAWINGS ARE SIMPSON STRONG-TIE CONNECTORS, UON. FASTENERS OF OTHER MANUFACTURERS MAY BE SUBSTITUTED PROVIDED THE LOAD VALUES OF THE SUBSTITUTED FASTENER FOR GROUP II WOOD SPECIES EQUALS OR EXCEEDS THE SPECIFIED FASTENER.

ALL NAILS SHALL BE COMMON WIRE NAILS UNLESS SHOWN OTHERWISE OR MANUFACTURE'S CONNECTOR LITERATURE SPECIFIES OTHERWISE.

NAILING OF ALL MEMBERS SHALL BE IN ACCORDANCE WITH THE BUILDING CODE. SEE CODE OR 4/S3.1 FOR TABLE.

ALL LUMBER USED IN EXTERIOR APPLICATIONS, INCLUDING: BALCONY DECK BOARDS, LEDGER, JOISTS, BEAMS, WOOD IN CONTACT WITH EXTERIOR MASONRY OR CONCRETE SLABS OR WALLS, AND SILL PLATES EXPOSED TO CONCRETE SHALL BE TREATED IN ACCORDANCE WITH AWPA U1. USE CATEGORY 2 FOR SILL PLATES, CATEGORY 3B FOR EXTERIOR MEMBERS, AND CATEGORY 4A FOR WOOD IN GROUND CONTACT. SEE AWPA U1 FOR ALL OTHER CASES.

ROOF SHEATHING SHALL BE 19/32" MINIMUM APA RATED SHEATHING, EXPOSURE 1 WITH 32/16 SPAN RATING. HOWEVER, 7/16" MINIMUM APA RATED SHEATHING, EXPOSURE 1 WITH 24/16 SPAN RATING MAY BE USED FOR ASPHALT SHINGLED OR STANDING SEAM METAL ROOFS.

ROOF SHEATHING SHALL BE NAILED WITH 8D NAILS AT 7/16" AND 1/2" DECK AND 10d NAILS AT 5/8" AND 3/4" DECK. SPACE NAILS AT 6" AT SUPPORTED EDGES OF DECK (4" @ EXTERIOR WALLS) AND 12" SPACING AT INTERMEDIATE SUPPORTS. AT GABLE ENDS, NAIL ROOF DECK AT 4" AT PANEL EDGES AND AT 6" AT INTERMEDIATE SUPPORTS FOR A DISTANCE OF 3'-0" FROM THE END WALL.

PROVIDE 2x4 BLOCKING FOR SUPPORT OF ROOF SHEATHING AT HIPS AND VALLEYS.

STRESS GRADE: SOUTHERN PINE NO. 2 OR ENGINEER APPROVED EQUAL. ALL DESIGN VALUES ARE UNDER NORMAL LOADING AND IN DRY CONDITIONS OF SERVICE.

PRESSURE-TREAT LUMBER IN ACCORDANCE WITH THE MANUAL OF RECOMMENDED PRACTICE OF THE AMERICAN WOOD PRESERVERS ASSOCIATION (AWPA).

ALL FASTENERS AND NAILS IN CONTACT WITH PRESSURE TREATED LUMBER SHALL BE MADE OF TYPE 304 OR TYPE 316 STAINLESS STEEL OR ASTM A653 TYPE G185 ZINC COATED STEEL UNLESS THE LUMBER IS TREATED WITH CCA, MCA, MCQ, uCA OR SBX (DOT), [BUT NOT SBX (DOT) WITH SODIUM SILICATE (NaSiO2)]. EXCEPT AT SWIMMING POOLS AND WITHIN 5 MILES OF SALT WATER STAINLESS STEEL MUST BE USED IF IN CONTACT WITH COPPER BASED PRESERVATIVES.

CONNECT OVER FRAMING (SUCH AS VALLEY TRUSSES) TO MAIN ROOF FRAMING BELOW WITH SIMPSON VTCR WITH (4) 10d NAILS INTO TRUSS AND (5) 10d x 1 1/2" NAILS INTO OVERFRAMING OR 1 1/4"x16 GA TWIST STRAP AT 48" MAX WITH (4) 10d NAILS EA END OF STRAP.

WOOD TRUSSES

TRUSS MANUFACTURER SHALL SUBMIT SHOP DRAWINGS INDICATING ACTUAL TRUSS LAYOUT, DESIGN, WIND UPLIFT AT BEARING LOCATIONS, NUMBER AND TYPES OF TRUSSES, ETC. SHOP DRAWINGS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER. TRUSS MANUFACTURER SHALL COORDINATE AND VERIFY ALL TRUSS DIMENSIONS AND DESIGNS WITH ARCHITECT'S DRAWINGS.

ROOF FRAMING PLAN AND TRUSS TYPES ARE DIAGRAMMATIC AND ARE INTENDED TO INDICATE DESIGN CONCEPT ONLY FOR ROOF CONFIGURATION.

TRUSSES SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH ANSI/TPI1 "NATIONAL DESIGN STANDARDS FOR METAL-PLATE-CONNECTED WOOD TRUSS CONSTRUCTION".

ROOF TRUSS DESIGN CRITERIA:	
LIVE LOAD	SEE DESIGN CRITERIA THIS SHEET
DEAD LOAD - STANDING SEAM ROOF	
	10 PSF BOT CHORD
MIN DEAD LOAD (FOR UPLIFT) - STANDING	SEAM ROOF8 PSF
WIND UPLIFT	PER CODE

BRACE BOTTOM CHORD AS REQUIRED FOR WIND UPLIFT.

COORDINATE TRUSS LOCATIONS/CONFIGURATION WITH PLUMBING WALLS AND HVAC EQUIPMENT SO AS TO AVOID CONFLICTS. SEE MECHANICAL DRAWINGS FOR EXACT LOCATIONS OF EQUIPMENT, DUCTS, STACKS, PIPES, ETC. GENERAL CONTRACTOR SHALL ENSURE TRUSS CONFIGURATION ACCOMMODATES ALL EQUIPMENT, DUCTS, ETC.

TEMPORARY TRUSS BRACING SHALL BE INSTALLED IN ACCORDANCE WITH "RECOMMENDED DESIGN SPECIFICATIONS FOR TEMPORARY BRACING OF METAL PLATE CONNECTED WOOD TRUSSES" (TPI-DSB) AND "COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES" (TPI-HIB). INSTALL ALL WEB BRACING REQUIRED BY THE TRUSS DESIGNER. TEMPORARY BOTTOM CHORD AND WEB BRACING SHALL REMAIN PERMANENTLY IN PLACE. THE BOTTOM CHORD BRACING SHALL NOT EXCEED 10' SPACING FOR TRUSSES WHERE NO SHEATHING IS ATTACHED TO THE TRUSS BOTTOM CHORD OR WITH TRUSS BOTTOM FILLER. PROVIDE 2x4 LATERAL BRACING AT 36" UNDER PIGGYBACK TRUSSES. ALL BRACING SHALL BE NAILED WITH (2) 16d NAILS TO TRUSSES.

AT TRUSSES REQUIRING HORIZONTAL WEB BRACING, PROVIDE 2x4 DIAGONAL BRACE (APPROX 45 DEGREES) AT 20' MAXIMUM SPACING. NAIL THE TOP END OF DIAGONAL TO WEB OF TRUSS AT ROOF. NAIL MIDDLE OF DIAGONAL TO TRUSS WEB AT HORIZONTAL LATERAL BRACING LOCATION AND THE BOTTOM END OF DIAGONAL TO BOTTOM OF WEB OF TRUSS AT CEILING.

SUPPLEMENTARY NOTES

PROVIDE ALL TEMPORARY BRACING, SHORING, GUYING OR OTHER MEANS TO AVOID EXCESSIVE STRESSES AND TO HOLD STRUCTURAL ELEMENTS IN PLACE DURING CONSTRUCTION. THE STRUCTURE SHOULD NOT BE CONSIDERED STABLE UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

IMEG CORP OR ANY OF ITS EMPLOYEES SHALL NOT HAVE CONTROL OR BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, PROCEDURES OR SEQUENCES FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR OR ANY OTHER PERSONS PERFORMING THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

VISUAL OBSERVATIONS OF THE STRUCTURAL SYSTEM BY IMEG CORP FOR GENERAL CONFORMANCE TO THE APPROVED PLANS AND SPECIFICATIONS DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY FOR THE INSPECTIONS REQUIRED BY THE INTERNATIONAL BUILDING CODE. VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR EMBEDS, OPENINGS, SLEEVES, ETC. NOT SHOWN ON THE STRUCTURAL DRAWINGS.

ALL STRUCTURAL OPENINGS AROUND OR AFFECTED BY MECHANICAL, ELECTRICAL AND PLUMBING EQUIPMENT SHALL BE VERIFIED WITH EQUIPMENT PURCHASED BEFORE PROCEEDING WITH STRUCTURAL WORK AFFECTED.

ANY ENGINEERING DESIGN PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW SHALL BEAR THE SEAL OF AN ENGINEER REGISTERED IN THE STATE OF THE PROJECT.

SHOP DRAWINGS AND SUBMITTALS

SHOP DRAWING SUBMITTALS ARE ONLY REVIEWED FOR GENERAL CONFORMANCE WITH THE INFORMATION SHOWN ON THE CONSTRUCTION DOCUMENTS. THE GENERAL CONTRACTOR MUST REVIEW AND APPROVE THE SHOP DRAWINGS PRIOR TO THEIR SUBMITTAL TO THE ENGINEER. SUBMITTALS WHICH DO NOT CONTAIN THE CONTRACTOR'S SHOP DRAWING STAMP SHALL BE RETURNED WITHOUT REVIEW. ANY REQUESTED CHANGES TO THE CONTRACT DOCUMENTS SHALL BE COMMUNICATED IN WRITING PRIOR TO SUBMITTING THE SHOP DRAWINGS AND CLOUDED ON THE SHOP DRAWINGS.

SHOP DRAWINGS MUST BE SUBMITTED FOR ENGINEER'S REVIEW OF THE FOLLOWING ITEMS: (S/S = SIGNED & SEALED SHOP DRAWING WITH CALCS, SD = SHOP DRAWING FOR REVIEW ONLY)

•	CONCRETE REINFORCING LAYOUT	S/SLI	SD	
•	CONCRETE MIX DESIGNS	S/S□	SD	
•	WOOD TRUSS SYSTEMS	S/S	SD□	
٠	MASONRY REINFORCEMENT LAYOUT	S/S□	SD	

COMPLETE SHOP DRAWINGS FOR CONSTRUCTION OF EACH BUILDING COMPONENT NOT DESIGNED BY THE DESIGN TEAM OF RECORD AND NOT SPECIFIED ON THE PROJECT CONSTRUCTION DOCUMENTS SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT AND SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF INSPECTION.

SOME STRUCTURAL SYSTEMS ARE DEFINED AS VENDOR-DESIGNED COMPONENTS PER THE STRUCTURAL DOCUMENTS. THESE ELEMENTS OF THE DESIGN ARE DEFERRED SUBMITTAL COMPONENTS AND HAVE NOT BEEN PERMITTED UNDER THE BASE BUILDING APPLICATION. VENDOR-DESIGNED COMPONENT SHOP DRAWINGS SHALL BE APPROVED BY THE COMPONENT DESIGNER ENGINEER PRIOR TO CURSORY REVIEW BY THE ENGINEER OF RECORD FOR LOADS IMPOSED ON THE BASE STRUCTURE. THE COMPONENT DESIGNER IS RESPONSIBLE FOR CODE CONFORMANCE AND ALL NECESSARY CONNECTIONS NOT SPECIFICALLY CALLED OUT ON ARCHITECTURAL OR STRUCTURAL DRAWINGS. SHOP DRAWINGS SHALL INDICATE MAGNITUDE AND DIRECTION OF ALL LOADS IMPOSED ON BASIC STRUCTURE. THE CONTRACTOR SHALL SUBMIT THE STAMPED COMPONENT SYSTEM DOCUMENTS TO THE BUILDING OFFICIAL FOR APPROVAL.

SPECIFICATIONS

CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 301. "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (LATEST EDITION). EXCEPT AS MODIFIED BY THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL EMPLOY AND PAY AN INDEPENDENT TESTING LABORATORY TO PERFORM CONCRETE TESTING.

MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO THE REQUIREMENTS OF "SPECIFICATIONS FOR MASONRY STRUCTURES - ACI 530.1/ASCE 6" (LATEST EDITION), EXCEPT AS MODIFIED BY REQUIREMENTS OF THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL EMPLOY AND PAY AN INDEPENDENT TESTING LABORATORY TO PERFORM MASONRY TESTING.

A GEOTECHNICAL ENGINEER SHALL BE EMPLOYED TO CONFIRM BEARING PRESSURE STATED PRIOR TO CONSTRUCTION. THE ENGINEER SHALL DEVELOP & ENSURE IMPLEMENTATION OF A SITE PREPARATION PROGRAM AS HE DEEMS NECESSARY TO ACHIEVE THE STATED BEARING PRESSURE.

FOOTING AND SLAB SUBGRADE PREPARATION SHALL BE IN COMPLIANCE WITH APPLICABLE REQUIREMENTS OF AUTHORITIES HAVING JURISDICTION.





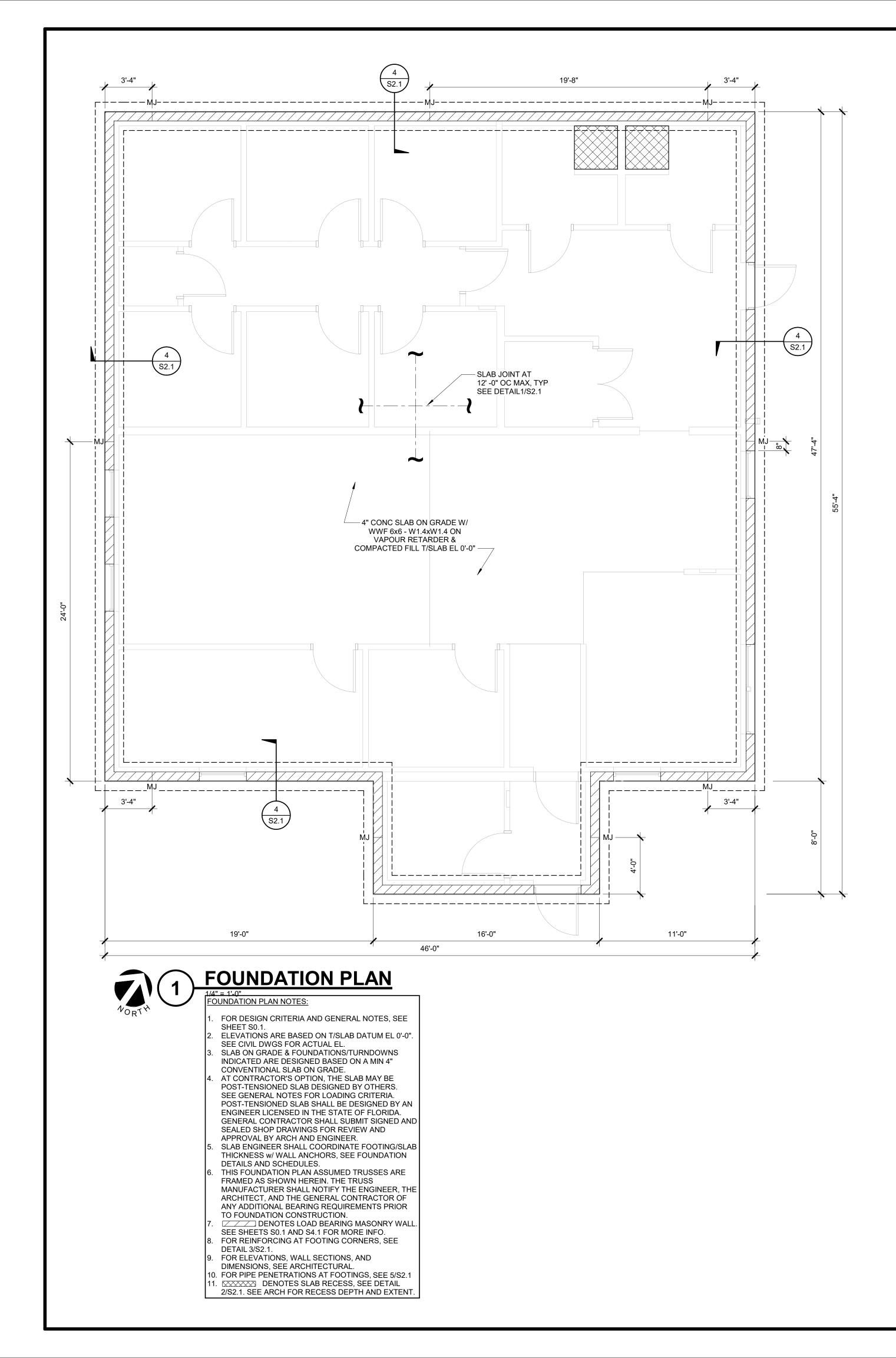
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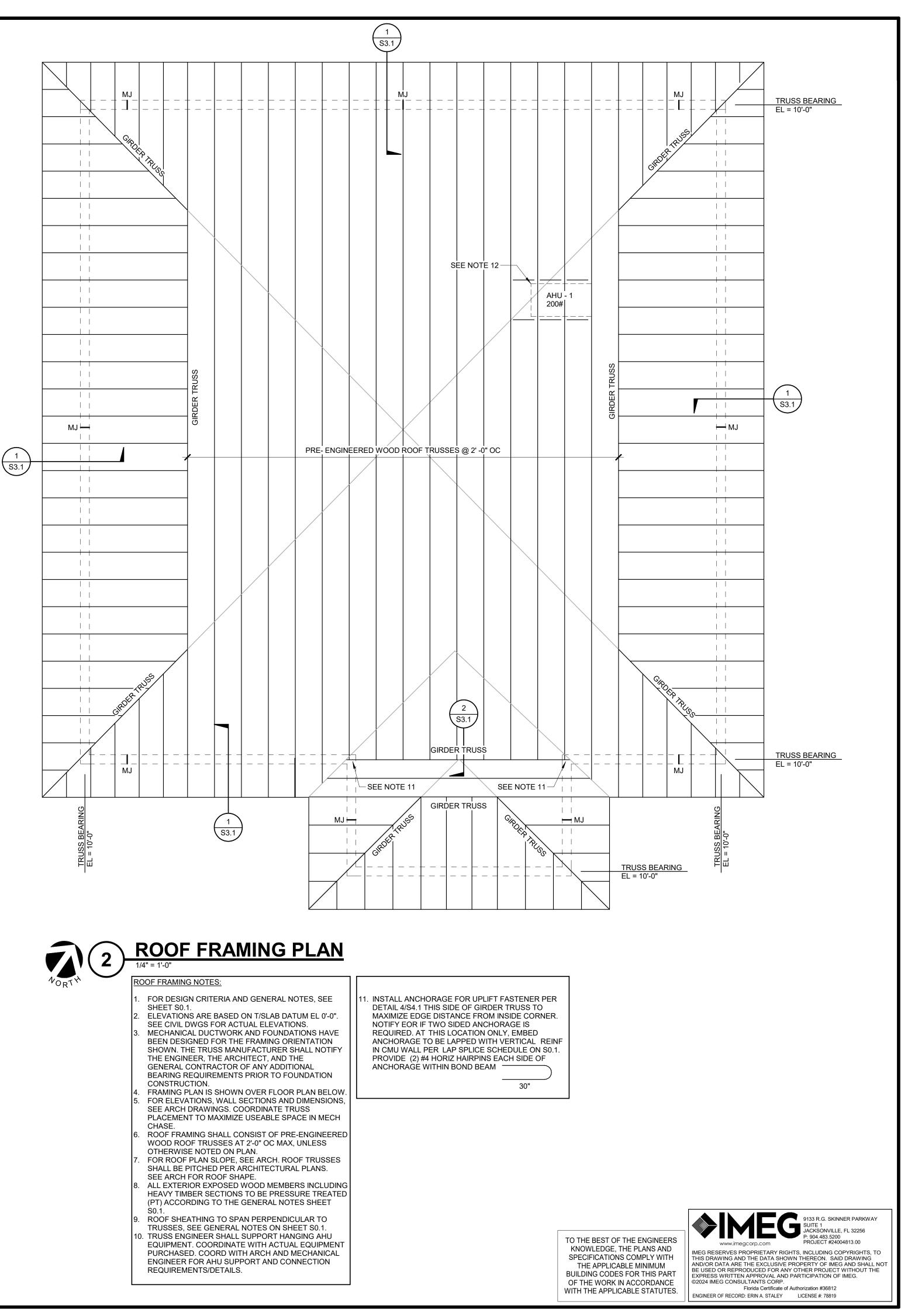
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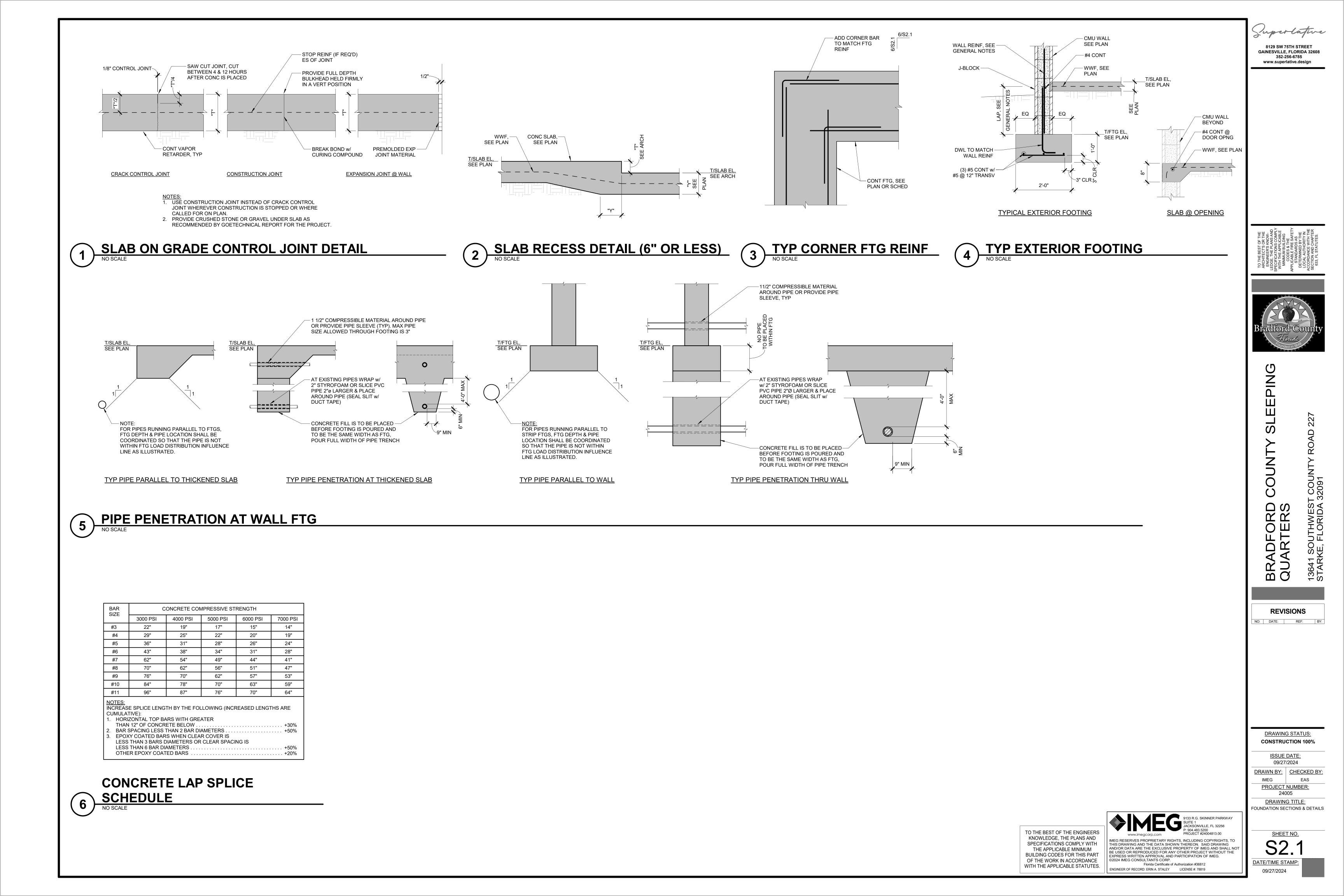
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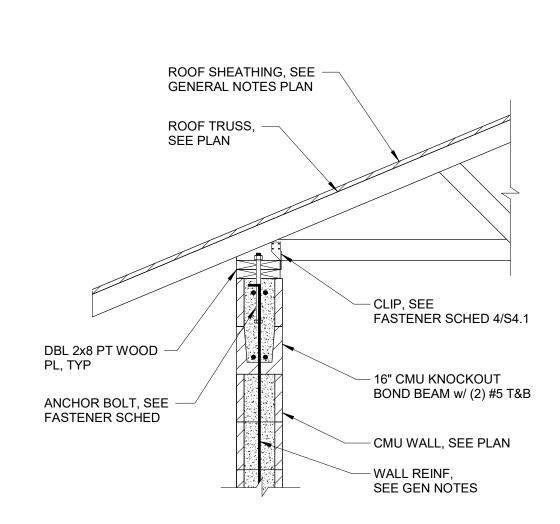
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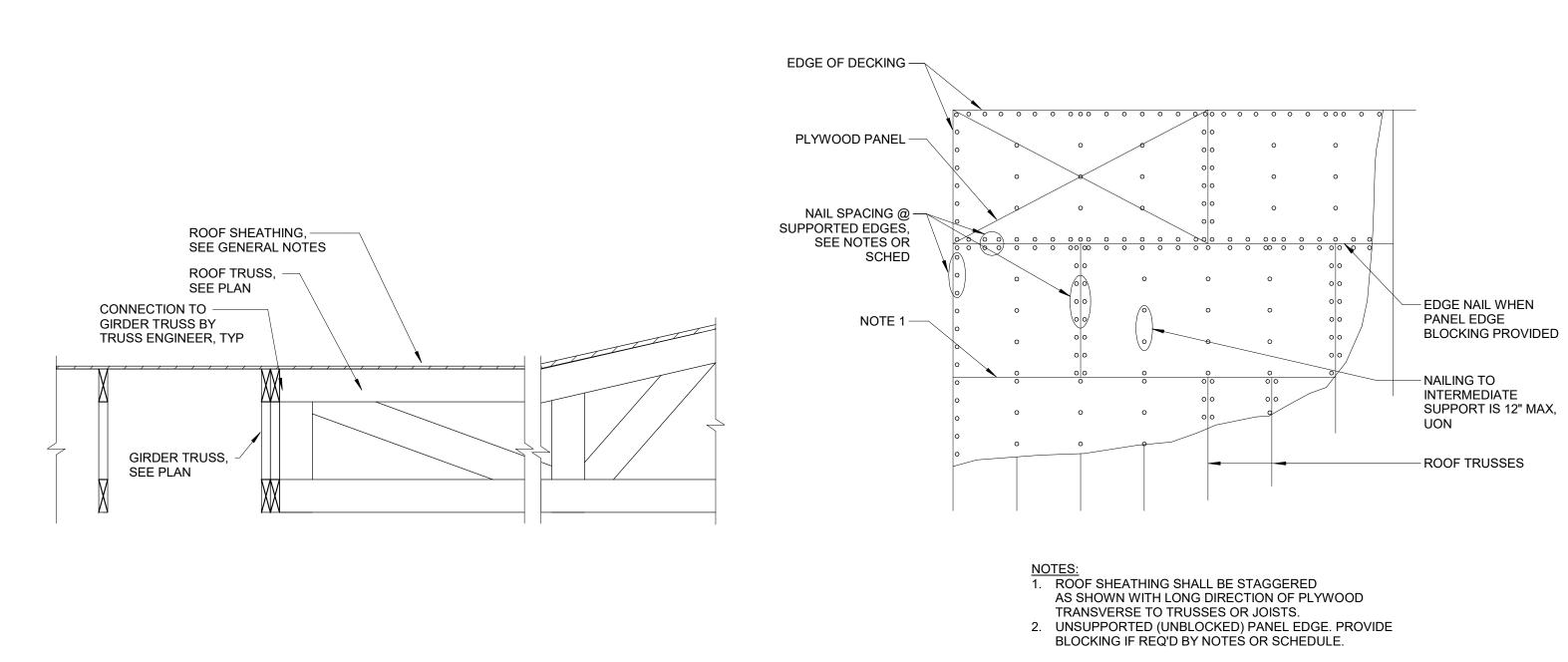












<u>NOTE:</u> TRUSS PROFILE SHOWN IS GENERIC COODINATE WITH ARCH.





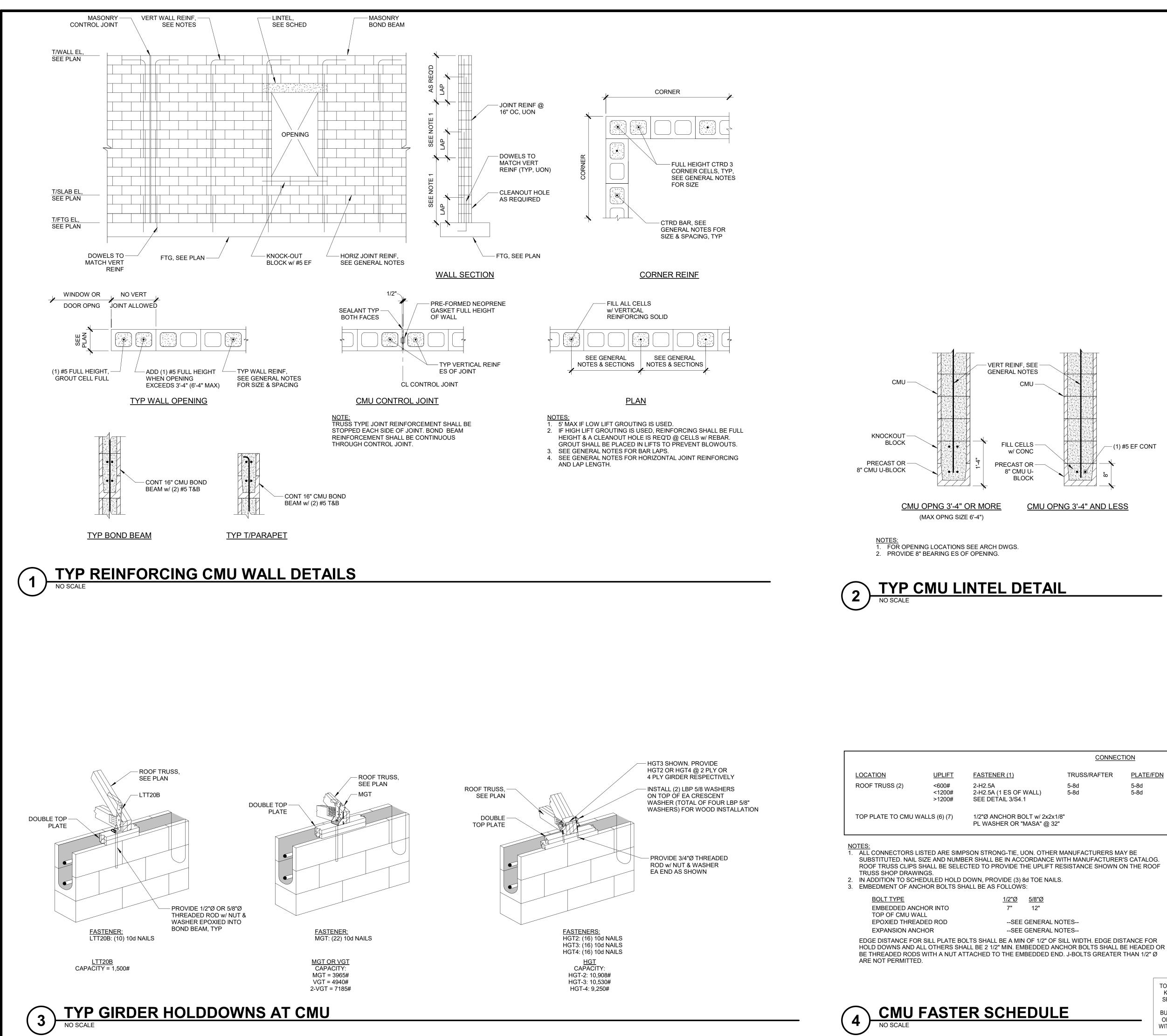
CONNECTION	NAIL LOCATION	NAIL (A)	GUN NAIL	STAPLE (B)
JOIST TO SILL OR GIRDER	TOENAIL	3-8d	3-3"x0.131"	3-3" 14 GA
BRIDGING TO JOIST	TOENAIL EA END	2-8d	2-3"x0.131"	2-3" 14 GA
1"x6" SUBFLOOR OR LESS TO EA JOIST	FACE NAIL	2-8d	-	-
WIDER THAN 1"x6" SUBFLOOR TO EA JOIST	FACE NAIL	3-8d	-	-
2" SUBFLOOR TO JOIST OR GIRDER	BLIND & FACE NAIL	2-16d	-	-
SOLE PLATE TO JOIST OR BLOCKING	TYPICAL FACE NAIL	16d @ 16"	3"x0.131" @ 8"	3" 14 GA @ 12"
TOP PLATE TO STUD	END NAIL	2-16d	3-3"x0.131"	3-3" 14 GA
STUD TO SOLE PLATE	TOENAIL	4-8d	4-3"x0.131"	3-3" 14 GA
STUD TO SOLE PLATE	END NAIL	2-16d	3-3"x0.131"	3-3" 14 GA
DOUBLE STUDS	FACE NAIL	16d @ 24"	3"x0.131" @ 8"	3" 14 GA @ 8"
DOUBLE TOP PLATES	TYPICAL FACE NAIL	16d @ 16"	3"x0.131" @ 12"	3" 14 GA @ 12"
DOUBLE TOP PLATES SPLICE (LAP 4'-0")	FACE NAIL	8-16d	12-3"x0.131"	12-3" 14 GA
BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	TOENAIL	3-8d	3-3"x0.131"	3-3" 14 GA
RIM JOIST TO TOP PLATE	TOENAIL	8d @ 6"	3"x0.131" @ 6"	3" 14 GA @ 6"
TOP PLATE INTERSECTIONS	FACE NAIL	2-16d	3-3"x0.131"	3-3" 14 GA
CONT HEADER, TWO PIECES	FACE NAIL	16d @ 16" T&B EDGE	-	-
CONT HEADER, THREE PIECES	FACE NAIL	16d @ 16" T&B EA FACE	-	-
CEILING JOISTS TO PLATE	TOENAIL	3-8d	5-3"x0.131"	5-3" 14 GA
CONT HEADER TO STUD	TOENAIL	4-8d	-	-
CEILING JOISTS, LAPS OVER PARTITIONS	FACE NAIL	3-16d MIN	4-3"x0.131"	4-3" 14 GA
CEILING JOISTS TO PARALLEL RAFTERS	FACE NAIL	3-16d MIN	4-3"x0.131"	4-3" 14 GA
RAFTER TO PLATE	TOENAIL	3-8d	3-3"x0.131"	3-3" 14 GA
1"x8" SHEATHING TO EA BEARING WALL	FACE NAIL	2-8d	-	-
WIDER THAN 1"x8" SHEATHING TO EA BRG	FACE NAIL	3-8d	-	-
BUILD-UP CORNER STUDS	FACE NAIL	16d @ 24"	3"x0.131" @ 16"	3" 14 GA @ 16"
2" PLANKS	@ EA BEARING	16d	-	-
COLLAR TIE TO RAFTER	FACE NAIL	3-10d	4-3"x0.131"	4-3" 14 GA
JACK RAFTER TO HIP	TOENAIL	3-10d	4-3"x0.131"	4-3" 14 GA
JACK RAFTER TO HIP	FACE NAIL	2-16d	3-3"x0.131"	3-3" 14 GA
ROOF RAFTER TO 2x RIDGE BEAM	TOENAIL	2-16d	3-3"x0.131"	3-3" 14 GA
ROOF RAFTER TO 2x RIDGE BEAM	FACE NAIL	2-16d	3-3"x0.131"	3-3" 14 GA
JOIST TO BAND JOIST	FACE NAIL	3-16d	5-3"x0.131"	5-3" 14 GA
LEDGER STRIP	FACE NAIL	3-16d	4-3"x0.131"	4-3" 14 GA

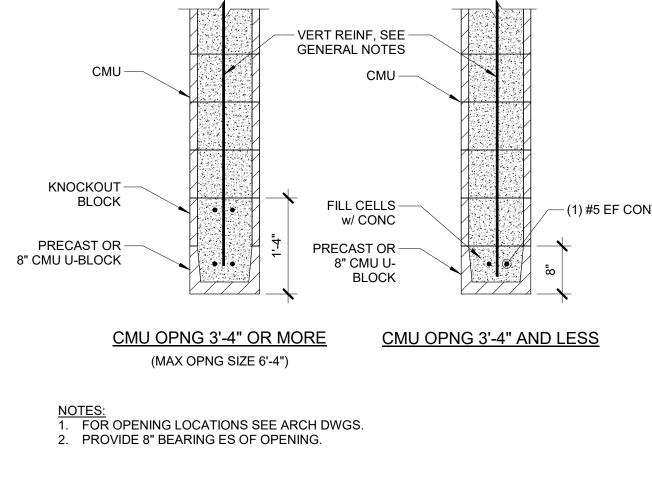
NOTES: A. COMMON WIRE NAILS ONLY ARE PERMITTED TO BE USED EXCEPT WHERE OTHERWISE STATED. B. STAPLES SHALL HAVE A MINIMUM CROWN WIDTH OF 7/16".

4 NAIL FASTENING SCHEDULE

3 NAILING REQUIREMENTS FOR ROOF SHEATHING NO SCALE

		Super-Lafine 8129 SW 75TH STREET GAINESVILLE, FLORIDA 32608 352-256-6785 www.superlative.design
		GAINESVILLE, FLORIDA 32608 352-256-6785
		REVISIONS NO: DATE: REF: BY:
		DRAWING STATUS: CONSTRUCTION 100% ISSUE DATE: 09/27/2024 DRAWN BY: CHECKED BY: IMEG EAS PROJECT NUMBER: 24005 DRAWING TITLE: SECTIONS & DETAILS
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	NT≺	NTY ROAD 227
		COUN ⁻ 2091
	RD RS	IWEST RIDA 3
	RADFORD CO UARTERS	13641 SOUTHWEST COU STARKE, FLORIDA 32091
	BRA QUA	13641 STARk
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		EF: BY:
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	09/27/2024 <u>DRAWN BY:</u> <u>CHE</u> IMEG	ECKED BY: EAS
	PROJECT NUMI 24005 DRAWING TIT MASONRY SECTIONS	<u>LE:</u>
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D DRAWING AND SHALL NOT WITHOUT THE IMEG.	54. DATE/TIME STAMP: 09/27/2024	
	J	

	<u>CONNECTI</u>	<u>ON</u>
	TRUSS/RAFTER	PLATE/FDN
	5-8d 5-8d	5-8d 5-8d
2x1/8") 32"		



904.483.5200

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